# CONCLUSIONS

- 1. HSC shows a larger strain-rate-sensitivity in compression than NSC as far as the peak stress and the strain are concerned, whereas in tension, both concretes show a similar strain-rate-sensitivity.
- 2. The experimental observations can be explained on the basis of a basic mechanism of the strain-rate-sensitivity (mainly the Stefan effect).
- 3. The long-term strength of HSC does not correspond to the so called 'critical stress' related to the maximum volume strain. It seems to be reasonable to consider the beginning of cracking inside the concrete as the critical point, which is shown by a significant increase of Poisson's ratio. According to the experimental analysis, the long-term strength of HSC is about 80% of its short-term strength.

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### REFERENCES

- P. H. Bischoff and S. H. Perry, "Compressive Behaviour of Concrete at High Strain Rates," <u>Materials and Structures</u>, Vol. 24, 1991, pp. 425-450.
- A. M. Neville, W. H. Dilger and J. J. Brooks, "Creep of Plain and Structural Concrete," <u>Construction Press</u>, London and New York, 1983, 361 pp.
- 3. E. J. Sellevold, "The Function of Condensed Silica Fume in High Strength Concrete," <u>Proceedings</u>, Symposium on Utilization of High Strength Concrete, Stavanger, Norway, June 15-18, 1987, pp. 39-49.
- 4. S. L. Sarkar and P. C. Aïtcin, "Comparative Study of the Microstructures of Normal and Very High-Strength Concretes," <u>Cement, Concrete and Aggregates</u>, CCAGDP, Vol. 9, No. 2, Winter 1987, pp. 57-64.

- A. Bentur, A. Goldman and M. D. Cohen, "The Contribution of the Transition Zone to the Strength of High Quality Silica Fume Concretes," <u>Bonding in Cementitious Composites</u>, S. Mindess and S. P. Shah, Editors, Materials Research Society, Boston, 1987, pp. 97-104.
- A. Goldman and A. Bentur, "Bond Effects in High-Strength Silica-Fume Concretes," <u>ACI Materials Journal</u>, Vol. 86, No. 5, September-October 1989, pp. 440-447.
- M. M. Smadi and F. O. Slate, "Microcracking of High and Normal Strength Concretes under Short- and Long-Term Loadings," <u>ACI</u> <u>Materials Journal</u>, Vol. 86, No. 2, March-April 1989, pp. 117-127.
- S. Nagataki and A. Yonekura, "Properties of Drying Shrinkage and Creep of High-Strength Concrete," <u>Transactions of the Japan Concrete</u> <u>Institute</u>, Vol. 4, 1982, pp. 223-236.
- J. C. Walraven, N. Han and J. Stroband, "Report on Various Tests on Concrete with High Strength," <u>CUR Commission C86</u>, TU Delft, April 1993 (in Dutch).
- N. Han, "Time-Dependent Behaviour of High-Strength Concrete Part I Literature Survey," <u>Stevin Laboratory Report</u>, No. 25.5-92-14, June 1992, Delft University of Technology, The Netherlands, 35 pp.
- N. Han, "Time-Dependent Behaviour of High-Strength Concrete Part II Experimental Descriptions and Results," <u>Stevin Laboratory Report</u>, No. 25.5-92-15, August 1992, Delft University of Technology, The Netherlands, 230 pp.
- N. Han, "Time-Dependent Behaviour of High-Strength Concrete Part III Experimental Analysis," <u>Stevin Laboratory Report</u>, No. 25.5-92-16, October 1992, Delft University of Technology, The Netherlands, 76 pp.
- P. Rossi, "A physical phenomenon which can explain the mechanical behaviour of concrete under high strain rates," <u>Materials and Structures</u>, Vol. 24, 1991, pp. 422-424.
- C. Rasch, "Spannungs-Dehnungs-Linien des Betons und Spannungsverteilung in der Biegedruckzone bei Konstanter Dehngeschwindigkeit," <u>Deutscher Ausschuss für Stahlbeton</u>, Heft 154, Berlin 1962, 72 pp.
- P. Stroeven, "Some Aspects of the Micromechanics of Concrete," <u>PhD</u> <u>Thesis</u>, Delft University of Technology, The Netherlands, 1973, 323 pp.

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- M. Held, "A contribution to the fabrication and design of compression members in HSC," <u>PhD Thesis</u>, Darmstadt University of Technology, Germany, 1992. (in German)
- 17. H. Rüsch, "Researches Toward a General Flexural Theory for Structural Concrete," <u>ACI Journal</u>, Vol. 57, July 1960, pp. 1-28.
- T. T. C. Hsu, F. O. Slate, G. M. Sturman and G. Winter, "Microcracking of Plain Concrete and Shape of the Stress-Strain Curve," <u>ACI Journal</u>, Vol. 60, No. 2, February 1963, pp. 209-224.
- S. P. Shah and S. Chandra, "Critical Stress, Volume Change, and Microcracking of Concrete," <u>ACI Journal</u>, Vol. 67, No. 10, October 1970, pp. 816-825.
- N. Han and J. C. Walraven, "The Effect of Strain Rate on the Compressive Properties of High Strength Concrete," <u>Progress in</u> <u>Concrete Research</u>, Annual Report, Vol. 2, 1991, Delft University of Technology, pp. 31-43.
- N. Han and J. C. Walraven, "Sustained Loading Effects in High-Strength Concrete," <u>Proceedings</u>, Symposium on Utilization of High Strength Concrete, Lillehammer, Norway, June 20-23, 1993, pp. 1076-1083.

Mixture	Unit	HSC	NSC	Note	
Cement	kg/m³	475	360	ENCI Portland Cement Class C	
Silica fume	kg/m³	25	0	dry powder	
W/(C+SF)		0.30	0.50	Water/(Cement+Silica Fume)	
Aggregate	kg/m³	1796	1816	total weight of aggregate	
0 - 4 mm 4 - 16 mm	kg/m³ kg/m³	718 1078	726 1090	Fine aggregate Coarse aggregate	
Superplasticizer	<b>%</b>	****	*****	percentage of weight of cement	
lignosulfonate lignosulfonate Naphtalene	96 96 96	0.2 0.6 2.2-2.5	0.2 (40% solid)	with a little Na-gluconate without Na-gluconate	
Air content	96	1	1	****	
Cube strength (28 days) 150×150×150 mm	N/mm <sup>2</sup>	113.8	42.3	Cube strength is determined at a constant loading rate of 13.5 KN/s	
	SD	4.1	1.4		
Prism strength (28 days) 100×100×400 mm	N/mm <sup>2</sup>	100	29.4	Prismatic strength is determined at a constant longitudinal strain rate of $7.3 \times 10^{-6} \text{ s}^{-1}$	
	SD	3.3	1.4		

#### TABLE 1 - MIX PROPORTIONS FOR HSC AND NSC

TABLE 2 — STATISTICAL ANALYSIS OF MECHANICAL PROPERTIES OF HSC AND NSC IN UNIAXIAL COMPRESSIVE TESTS AT VARIOUS STRAIN RATES

G/S	G/S de, / dt		$\epsilon_{\rm ymax}$ ( $\times 10^{-3}$ )		σ <sub>m-γ</sub> /σ <sub>σωx</sub>	
(No. of samples)	(×10 <sup>-6</sup> s <sup>-1</sup> )	item	HSC	NSC	HSC	NSC
R1	6.250	average	1.514	0.389	0.960	0.810
(3)		SD	0.021	0.036	0.017	0.056
R2	2.500	average	1.438	0.426	0.957	0.926
(2)		SD	0.011	0.029	0.006	0.048
<b>R</b> 3	0.430	average	1.356	0.447	0.943	0.952
(2)		SD	0.063	0.042	0.057	0.037
R4	0.190	average	1.431	0.387	0.942	0.865
(2)		SD	0.000	0.009	0.012	0.058
R5	0.000	average	1.469	0.456	0.922	0.761
(2)	0.020	SD	0.062	0.031	0.009	0.028
R6	0.010	average	1.485	0.410	0.928	0.874
(2)		SD	0.045	0.006	0.013	0.080

 $\varepsilon_{vmax}$  means the maximum volume strain.

 $\sigma_{mv}$  means the stress at the maximum volume strain.



Fig. 1-Setup of measurements for: a) compressive tests; and b) tensile tests



Fig. 2—Basic mechanism of strain-rate sensitivity



Fig. 3-Typical failure modes



Fig. 4—Stress-strain curves for HSC and NSC in uniaxial compressive tests at various transverse strain rates



Fig. 5—Stress-deformation curves for HSC and NSC in uniaxial tensile tests at various longitudinal strain rates



Fig. 6—Peak stresses versus transverse strain rates for HSC and NSC in uniaxial compressive tests



Fig. 7—Peak stresses versus longitudinal strain rates for HSC and NSC in uniaxial tensile tests



