

(gradual softening) branches.

Measurement of load-shear displacement curve--On the other hand, the relation between load and shear displacement at load end can be also used to identify the local $\tau - \delta$ relation if the shape such as the ones shown in Fig. 4 is assumed a priori. The load-shear displacement relation at loading end as shown in Fig. 16 may be measured directly with the help of clip gauge (Fig. 3). Moreover, it can be also obtained through the measurement of FRP strain distribution for whole loading steps, where the shear displacement can be calculated through the integration of strain distribution along FRP laminate. Fig. 16 shows some special points such as occurrence of micro debonding (A), occurrence of macro debonding (B) and occurrence of final debonding failure (C). Both points (a) (u_1 , P_1) and (B) (u_2 , $P_{\max}$) can be considered to correspond to the ones (δ_1 , $\tau_{\max}$) and (δ_f , 0) in $\tau - \delta$ curve. Therefore, the following expressions can be obtained easily.

$$\delta_f = u_2, \tau_f = \frac{P_{\max}^2}{E_1 t_1 b_1^2 u_2}, \delta_1 = u_1 \text{ or } \delta_1 = \frac{P_1^2}{E_1 t_1 b_1^2 \tau_f} \quad (13)$$

A redundant equation can be used to verify the accuracy. After occurrence of macro debonding, the interfacial cracking starts to propagate to the FRP end. Theoretically, there is no loading increase or decrease during the debonding propagation from (B) to (C). However, due to the heterogeneities along the FRP-concrete interface, both case 1 and case 2 as shown in Fig. 16 is observed in the experiment. Fig. 17 shows the identified result for the specimen S-CFS-400-25, i.e. $\delta_1 = 0.1\text{mm}$, $\delta_f = 0.3\text{mm}$, $\tau_f = 8\text{MPa}$. The load-shear displacement curve calculated theoretically based on the identified parameters is also shown in Fig. 17 for comparison with the ones based on measurement of clip gauge and strain gauge of FRP strain distribution. The $\tau - \delta$ relation identified by this kind of approach is considered to be more practical.

CONCLUSIONS

From both experimental and analytical studies on bond properties of FRP laminates the following conclusions are obtained

- 1) Through the experimental program using both single-lap shear test specimens and double-lap shear test specimens designed to study the interfacial behavior of FRP-concrete interface, the load-carrying capacity, FRP strain distribution, interfacial shear stress distribution and effective stress transfer length affected by the equivalent stiffness of FRP laminates are quantitatively discussed.

- 2) The nonlinear relationships derived for some typical local shear-deformation curves with and without softening behavior are used to discuss the shear stress transfer and fracture propagation behavior by comparing with experimental findings. It is found that the ascending and descending model gives more close simulation for both interfacial shear stress distribution and effective stress transfer length.
- 3) Through the comparison between experimental and theoretical results, the interfacial fracture energies are quantified.
- 4) The two methods to determine the local shear stress-displacement relationship are discussed, based on the following experimental results, respectively: a) Load-shear displacement relation at loading end; b) FRP strain distribution, through which the local shear-displacement curve can be obtained from the relationship of shear stresses varying with locations along the interface at a given appropriate loading step or with loading step at a given appropriate locations. The identified results by method a) is considered to be practical.

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TABLES

Table 1 Mechanical properties of FRP laminates and concrete

Materials		Specific Characteristics	Properties	Fiber Arial weight g/m ²	Design Thickness mm
Concrete		Comp. Strength (Single-lap Shear Test)	57.6 MPa	-	-
		Comp. Strength (Double-lap Shear Test)	42.0 MPa	-	-
Carbon Fiber Sheet	HT	Young's Modulus	230 GPa	150	0.083
		Tensile strength	4.2 GPa	200	0.111
				300	0.167
	HM	Young's Modulus	390 GPa	300	0.167
PBO Fiber Sheet	HM	Tensile strength	4.4 GPa		
Carbon Fiber Plate	HT	Young's Modulus	270 GPa	200	0.128
		Tensile strength	4.5 GPa		
		Young's Modulus	138 GPa	1100	1.0
		Tensile strength	2.52 GPa	2200	2.0

Table 2 Specimens used for shear tests

(a) Double-lap shear test

Quality of FRP laminate	Stiffness of FRP kN/mm	Fiber Arial weight g/m ²	Concrete Surface Preparation	Bond Length L mm	No. of Specimens
HT Carbon Fiber Sheet	19.1	150	Diamond sander/putty	300	D-CFS-150-30
	38.4	300	same	300	D-CFS-300-30
	76.8	600	same	300	D-CFS-600-30
HM Carbon Fiber Sheet	64.4	300	same	300	D-CFM-300-30
Aramid Fiber Sheet	23.9	280	same	300	D-AR-280-30

(b) Single-lap shear test

Quality of CFS	Stiffness of CFS kN/mm	Fiber Arial weight g/m ²	Concrete Surface Preparation	Bond Length L mm	No. of Specimens
HT Carbon Fiber Sheet	51.1	400	Diamond sander/putty	250	S-CFS-400-25
HM Carbon Fiber Sheet	64.4	300	same	250	S-CFM-300-25
	193.1	900	same	250	S-CFM-900-25
HM PBO Fiber Sheet	34.6	200	same	250	S-PBO-200-25
HT Carbon Fiber Plate	138.0	1100	same	250	CP1-L25
	276.0	2200	same	250	CP2-L25

Table 3 Shear test results

(a) Double-lap shear test

Quality of FRP laminate	Stiffness of FRP kN/mm	Bond Length L mm	Specimens	No. of Specimen	Max. Load kN	Max. Debonding Load kN/cm	Failure Mode
HT HT HT	19.1	300	D-CFS-150-30	1	24.4	2.44	Debonding
				2	23.6	2.36	Debonding
				3	24.5	2.45	Debonding
				Av.	24.1	2.41	-
	38.4	300	D-CFS-300-30	1	37.8	3.78	Rupture of FRP
				2	33.9	3.39	Debonding
				3	33.3	3.33	Debonding
				Av.	35.0	3.50	-
	76.8	300	D-CFS-600-30	1	51.3	5.13	Debonding
				2	50.7	5.07	Debonding
				3	54.5	5.45	Debonding
				Av.	52.1	5.21	-
HM Carbon Fiber	64.4	300	D-CFM-300-30	1	39.0	3.90	Rupture of FRP
				2	39.0	3.90	Debonding
				3	32.5	3.25	Rupture of FRP
				Av.	36.8	3.68	-
Aramid Fiber	23.9	300	D-AR-280-30	1	25.5	2.55	Debonding
				2	25.7	2.57	Debonding
				3	23.8	2.38	Debonding
				Av.	25.0	2.50	-

(b) Single-lap shear test

Quality of FRP Laminate	Stiffness of FRP kN/mm	Bond Length L mm	Specimens	No. of Specimen	Max. Load kN	Max. Debonding Load kN/cm	Failure Mode
HT Carbon Fiber Sheet	51.1	250	S-CFS-400-25	1	15.4	3.84	Debonding
				2	13.9	3.48	Debonding
				3	13.0	3.25	Debonding
				Av.	14.1	3.52	-
HM Carbon Fiber Sheet	64.4	250	S-CFM-300-25	1	12.0	3.01	Debonding (within adhesive)
				2	11.9	2.97	Debonding (within adhesive)
				Av.	12.0	2.99	-
	193.1	250	S-CFM-900-25	1	25.9	6.48	Debonding
				2	23.4	5.85	Debonding
				3	23.7	5.92	Debonding
				Av.	23.5	5.88	-
HM PBO Fiber Sheet	34.6	250	S-PBO-200-25	1	14.3	3.56	Debonding
				2	12.2	3.04	Debonding
				Av.	13.2	3.30	-
HT Carbon Fiber Plate	138.0	250	CP1-L25	1	16.99	3.40	Debonding
				2	16.96	3.39	Debonding
				3	17.23	3.45	Debonding
				Av.	17.06	3.41	-
	276.0	250	CP2-L25	1	22.32	4.46	Debonding
				2	23.69	4.74	Debonding
				3	23.11	4.62	Debonding
				Av.	23.04	4.61	-

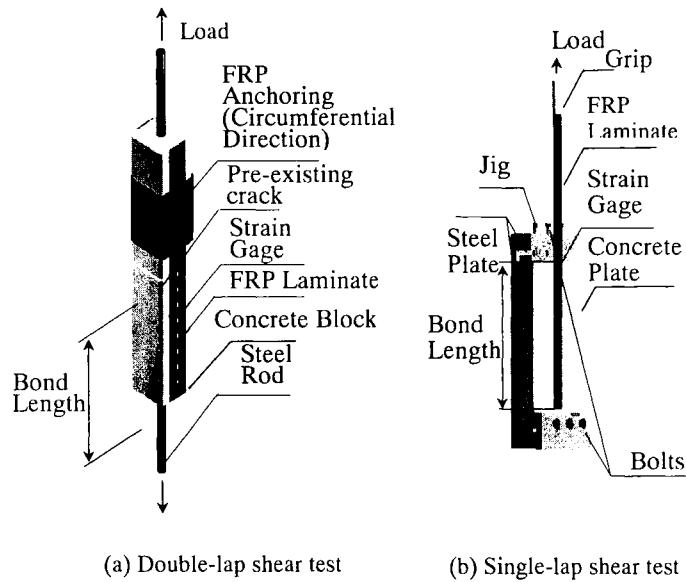


Fig. 1. Experimental methods

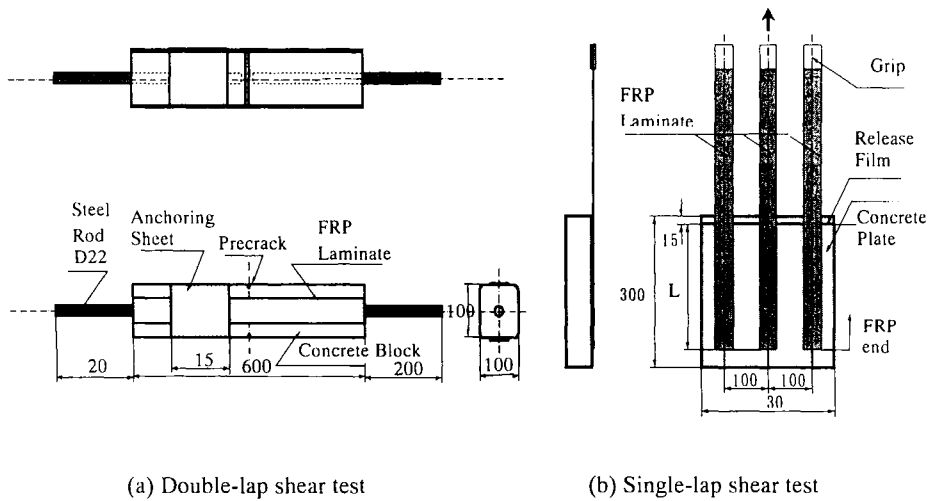


Fig. 2. Specimen configuration

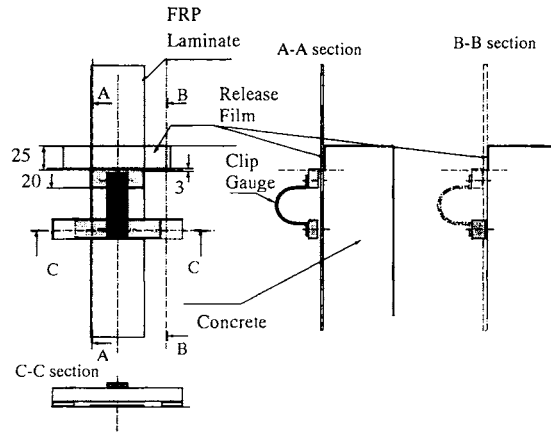


Fig. 3. Measurement of relative shear displacement

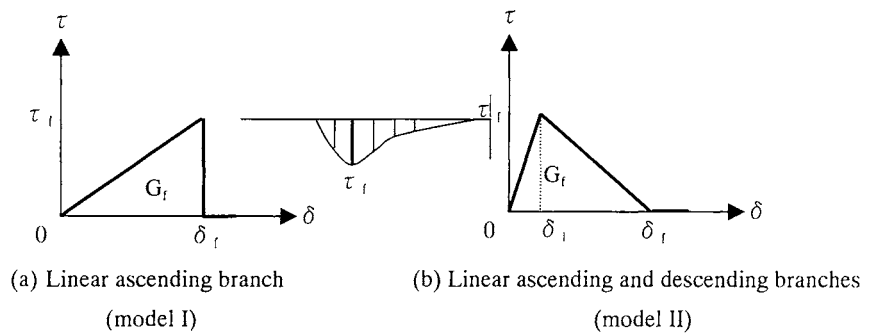


Fig. 4. Simplified local shear stress-slip relations

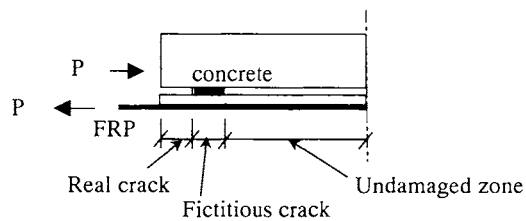


Fig. 5. Fictitious crack model and shear stress distribution

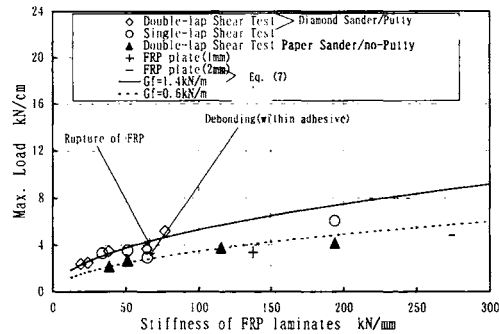


Fig. 6. Relationship between load-carrying capacity and FRP stiffness

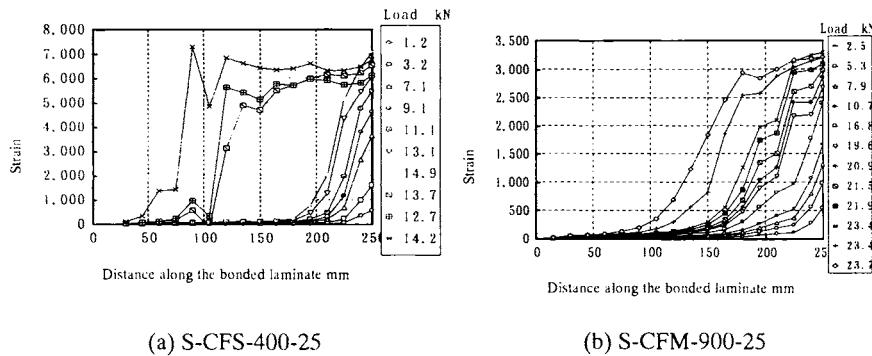


Fig. 7. Examples of strain distribution in FRP laminate

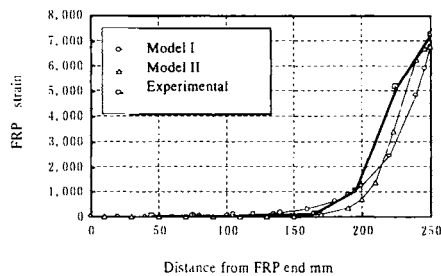


Fig. 8. Comparison of experimental and theoretical FRP strain distribution

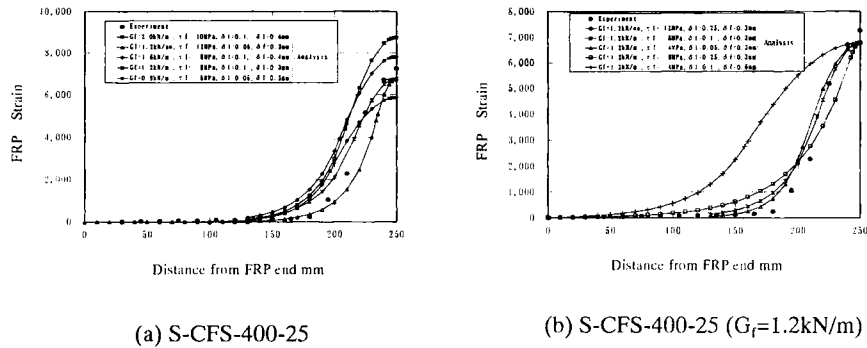


Fig. 9. Identification of FRP strain distribution

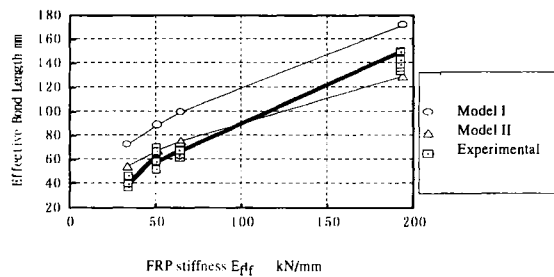


Fig. 10. Comparison of experimental and theoretical results on effective stress transfer length

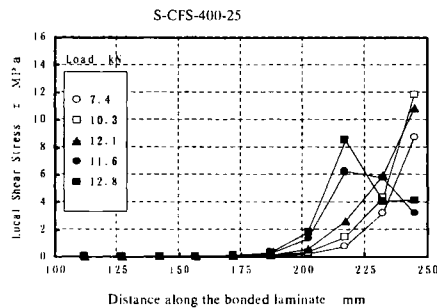


Fig. 11. Experimental results on local shear stress distribution

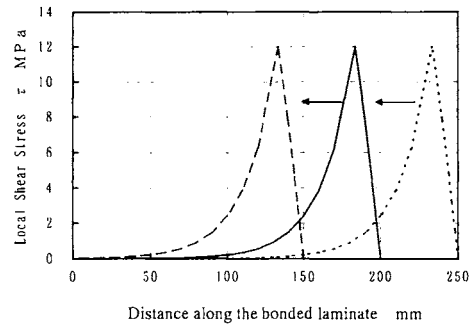


Fig. 12. Shear stress distribution due to crack propagation (theoretical value)

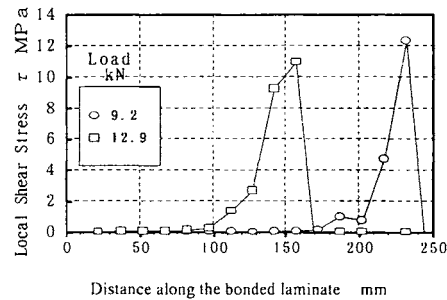


Fig. 13. Shear stress distribution due to crack propagation (experimental value)

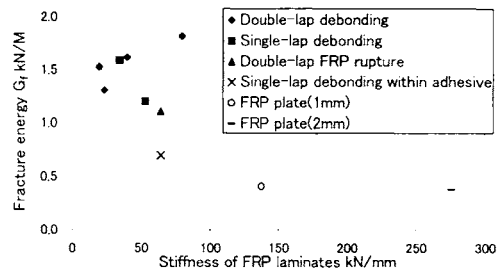


Fig. 14. Identified results of interfacial fracture energy