

TABLE 6—PROPERTIES OF FRESH CONCRETE
FOR BEAM SHEAR TESTS

<u>Mixture</u>	<u>No. of Batches</u>	<u>No. of Specimens</u>	<u>Slump in. (mm)</u>	<u>Unit Weight lbs/cu ft (kgs/cu m)</u>	<u>Air Content %</u>
LWC1	3	27	5.6 (142)	116.6 (1868)	4.8
LWC2	3	27	5.1 (130)	112.4 (1801)	5.8
HSLWC	3	27	5.1 (130)	128.4 (2057)	4.4

Note: Nine 6 by 12 in. (150 by 300 mm) cylinders were made from each batch. One batch was used to cast each shear beam test specimen.

TABLE 8—PROPERTIES OF FRESH CONCRETE
FOR SHEAR FRICTION TEST SPECIMENS

<u>Mixture</u>	<u>Batch No.</u>	<u>Slump in. (mm)</u>	<u>Air Content %</u>	<u>Unit Weight lbs/cu ft (kgs/cu m)</u>	
LWC1	1	5.9 (150)	4.6	115.2	(1846)
	2	5.3 (135)	5.4	115.6	(1852)
	3	<u>5.9 (150)</u>	<u>5.0</u>	<u>115.6</u>	<u>(1852)</u>
	Avg.	5.7 (145)	5.0	115.5	(1850)
LWC2	1	5.6 (142)	5.6	113.8	(1823)
	2	6.5 (165)	6.4	111.4	(1785)
	3	<u>5.7 (145)</u>	<u>6.0</u>	<u>112.3</u>	<u>(1799)</u>
	Avg.	5.9 (150)	5.0	112.5	(1802)
HSLWC	1	8.1 (206)	4.5	127.5	(2043)
	2	5.0 (127)	4.5	128.7	(2062)
	3	<u>8.3 (211)</u>	<u>4.2</u>	<u>127.7</u>	<u>(2046)</u>
	Avg.	7.1 (180)	4.3	128.0	(2051)

TABLE 7—SUMMARY OF BEAM SHEAR TEST RESULTS

Mixture	Spec. No.	28-day Moist Cured Compressive Strength, f'_c		Compressive Strength at Test, f_c		Splitting Tensile Strength, f_{ct}		V_{cr}^*		V_u^{**}	
		psi	(MPa)	psi	(MPa)	psi	(MPa)	psi	(MPa)	psi	(MPa)
LWC1	1	8300	(57.2)	8750	(60.3)	480	(3.3)	230	(1.6)	520	(3.6)
	2	8240	(56.8)	8700	(60.0)	420	(2.9)	240	(1.7)	530	(3.7)
	3	8160	(56.3)	8290	(57.2)	420	(2.9)	210	(1.4)	280	(1.9)
LWC2	1	8340	(57.5)	8800	(60.7)	440	(3.0)	230	(1.6)	530	(3.7)
	2	8730	(60.2)	8720	(60.1)	410	(2.8)	230	(1.6)	520	(3.6)
	3	9280	(64.0)	9440	(65.1)	420	(2.9)	210	(1.4)	430	(3.0)
HSLWC	1	10910	(75.2)	10980	(75.7)	540	(3.7)	240	(1.7)	520	(3.6)
	2	11110	(76.6)	10360	(71.4)	550	(3.8)	250	(1.7)	400	(2.8)
	3	10740	(74.1)	11120	(76.7)	440	(3.0)	280	(1.9)	700	(4.8)

* V_{cr} = nominal average shear stress at inclined cracking at each end of beam.
 = $P_{cr}/2bd$, where P_{cr} = total load on beam at occurrence inclined cracking;
 b = 6 in. (152 mm); d = 10 in. (254 mm).

** V_u = nominal shear stress at maximum load.
 = $P_u/2bd$, where P_u = total maximum load on beam; b = 6 in. (152 mm);
 d = 10 in. (254 mm).

TABLE 9—PROPERTIES OF HARDENED CONCRETE FOR SHEAR FRICTION TEST SPECIMENS

Mixture	Batch No.	Shear Friction Specimen No.	Compressive Strength		Splitting Tensile Strength At Test**
			28-days Age*	At Test**	
			psi (MPa)	psi (MPa)	psi (MPa)
LWC1	1	1 and 4	8120 (56.0)	8490 (58.5)	--- (----)
	2	2 and 5	8410 (58.0)	8510 (58.7)	--- (----)
	3	3 and 6	<u>8300 (57.2)</u>	<u>8290 (57.2)</u>	--- (----)
	Avg.		8270 (57.0)	8430 (58.1)	--- (----)
LWC2	1	1 and 4	9110 (62.8)	9270 (63.9)	420 (2.90)
	2	2 and 5	8610 (59.4)	8760 (60.4)	400 (2.76)
	3	3 and 6	<u>8530 (58.8)</u>	<u>8730 (60.2)</u>	<u>370 (2.55)</u>
	Avg.		8750 (60.3)	8920 (61.5)	400 (2.76)
HSLWC	1	1 and 4	10750 (74.1)	10310 (71.1)	490 (3.38)
	2	2 and 5	10940 (75.4)	10910 (75.2)	530 (3.65)
	3	3 and 6	<u>10620 (73.2)</u>	<u>11020 (76.0)</u>	<u>500 (3.45)</u>
	Avg.		10770 (74.3)	10750 (74.1)	510 (3.52)

* Moist cured.

** Moist cured for 14-days, then cured in air until tested. Shear Friction tests of Specimens 3 and 6 of both LWC1 and LWC2 were tested at 28-days age. All other specimens were tested at 29-days age.

TABLE 10—SUMMARY OF SHEAR FRICTION TEST RESULTS

Mixture	Specimen Number	A_{vf}		f_y^*		ρf_y		Average Initial Crack Width		Maximum Stress V_u^{**}	$V_u/\rho f_y$	μ^{***}
		sq in.	(sq mm)	ksi (MPa)	psi (MPa)	psi (MPa)	in. (mm)	in. (mm)	psi (MPa)			
LWC1	1	0.44	(284)	53.6 (370)	281 (1.94)	281 (1.94)	0.010 (0.25)	0.010 (0.25)	287 (1.98)	1.02	1.19	
	2	0.44	(284)	53.6 (370)	281 (1.94)	281 (1.94)	0.013 (0.33)	0.013 (0.33)	365 (2.52)	1.30	1.19	
	3	0.44	(284)	53.6 (370)	281 (1.94)	281 (1.94)	0.011 (0.28)	0.011 (0.28)	413 (2.85)	1.47	1.19	
	4	0.80	(516)	68.0 (469)	648 (4.47)	648 (4.47)	0.010 (0.25)	0.010 (0.25)	761 (5.25)	1.17	1.19	
	5	0.80	(516)	69.0 (476)	657 (4.53)	657 (4.53)	0.012 (0.30)	0.012 (0.30)	680 (4.69)	1.04	1.19	
	6	0.80	(516)	68.0 (469)	648 (4.47)	648 (4.47)	0.012 (0.30)	0.012 (0.30)	727 (5.01)	1.12	1.19	
LWC2	1	0.44	(284)	53.6 (370)	280 (1.93)	280 (1.93)	0.008 (0.20)	0.008 (0.20)	489 (3.37)	1.75	1.19	
	2	0.44	(284)	53.6 (370)	280 (1.93)	280 (1.93)	0.011 (0.28)	0.011 (0.28)	335 (2.31)	1.20	1.19	
	3	0.44	(284)	53.6 (370)	280 (1.93)	280 (1.93)	0.010 (0.25)	0.010 (0.25)	299 (2.06)	1.07	1.19	
	4	0.80	(516)	68.0 (469)	648 (4.47)	648 (4.47)	0.010 (0.25)	0.010 (0.25)	739 (5.10)	1.14	1.19	
	5	0.80	(516)	68.5 (472)	652 (4.50)	652 (4.50)	0.010 (0.25)	0.010 (0.25)	692 (4.77)	1.06	1.19	
	6	0.80	(516)	68.5 (472)	652 (4.50)	652 (4.50)	0.011 (0.28)	0.011 (0.28)	680 (4.69)	1.14	1.19	
HSLWC	1	0.44	(284)	72.1 (497)	378 (2.61)	378 (2.61)	0.009 (0.23)	0.009 (0.23)	668 (4.61)	1.77	1.19	
	2	0.44	(284)	72.1 (497)	378 (2.61)	378 (2.61)	0.008 (0.20)	0.008 (0.20)	548 (3.78)	1.45	1.19	
	3	0.44	(284)	72.1 (497)	378 (2.61)	378 (2.61)	0.010 (0.25)	0.010 (0.25)	585 (4.03)	1.55	1.19	
	4	0.80	(516)	66.8 (461)	636 (4.39)	636 (4.39)	0.010 (0.25)	0.010 (0.25)	870 (6.00)	1.37	1.19	
	5	0.80	(516)	66.8 (461)	636 (4.39)	636 (4.39)	0.011 (0.28)	0.011 (0.28)	870 (6.00)	1.37	1.19	
	6	0.80	(516)	66.8 (461)	636 (4.39)	636 (4.39)	0.009 (0.23)	0.009 (0.23)	894 (6.16)	1.41	1.19	

* Yield stress of transverse reinforcement as determined by test.

** Based on maximum measured load prior to rapid increases in slip.

*** ACI 318-89 for sanded lightweight concrete where $\mu = 1.4\lambda$ and $\lambda = 0.85$.

TABLE 11—PROPERTIES OF FRESH CONCRETE
FOR BEARING TEST SPECIMENS

<u>Mixture</u>	<u>Batch No.</u>	<u>Slump</u>		<u>Air Content</u> %	<u>Unit Weight</u>	
		<u>in.</u>	<u>(mm)</u>		<u>lbs/cu ft</u>	<u>(kgs/cu m)</u>
LWC1	1	5.8	(147)	6.0	114.6	(1838)
	2	5.0	(127)	5.3	116.7	(1870)
	3	5.4	(137)	5.6	115.6	(1852)
	4	<u>5.5</u>	<u>(140)</u>	<u>5.7</u>	<u>115.5</u>	<u>(1850)</u>
	Avg.	5.4	(137)	5.6	115.6	(1852)
LWC2	1	5.8	(147)	5.4	115.5	(1850)
	2	5.9	(150)	5.1	116.0	(1858)
	3	5.9	(150)	5.6	116.5	(1866)
	4	<u>5.3</u>	<u>(135)</u>	<u>5.7</u>	<u>115.6</u>	<u>(1852)</u>
	Avg.	5.7	(145)	5.4	115.9	(1857)

TABLE 13—PROPERTIES OF CONCRETE FOR
REINFORCING BAR DEVELOPMENT LENGTH TESTS

<u>Mixture</u>	<u>Unhardened Concrete</u>			<u>Compressive Strength</u>	
	<u>Slump</u> <u>in.</u> <u>(mm)</u>	<u>Air</u> <u>Content</u> <u>%</u>	<u>Unit Weight</u> <u>lbs/cu ft</u> <u>(kgs/cu m)</u>	<u>28 Days</u> <u>psi*</u> <u>(MPa)</u>	<u>At Test</u> <u>psi**</u> <u>(MPa)</u>
LWC1	5.4 (137)	5.2	117.3 (1879)	8970 (61.8)	8900 (61.4)
LWC2	5.8 (147)	5.1	114.8 (1839)	9020 (62.2)	9320 (64.3)

* 28 days moist cured.

** 14 days moist cured followed by curing in air until tested.

TABLE 12—SUMMARY OF BEARING TEST RESULTS

Mixture	Batch No.	Bearing Test Specimen No.	28-Day Moist Cured Compressive Strength* psi (MPa)	At Test**			Bearing Test Observations			
				Age at Test days	Compressive Strength* psi (MPa)	Splitting Tensile Strength* psi (MPa)	Load at First Cracking*** (kips) (kN)		Maximum Load**** (kips) (kN)	
LWC1	1	1	8720 (60.1)	7	8030 (55.4)	410 (2.83)	600 (2667)		774 (3443)	
	2	2		7			600 (2667)		810 (3603)	
	3	3	8950 (61.7)	28	8800 (60.7)	480 (3.31)	650 (2891)		852 (3790)	
	4	4		28			650 (2891)		889 (3954)	
LWC2	1	1	8630 (59.5)	7	7800 (53.8)	400 (2.76)	600 (2667)		780 (3469)	
	2	2		7			550 (2446)		748 (3327)	
	3	3	8790 (60.6)	28	8810 (60.7)	430 (2.96)	600 (2667)		828 (3683)	
	4	4		28			650 (2891)		804 (3576)	

* Average of three 6 by 12 in. (150 by 300 mm) cylinders.

** Specimens tested 7 days after fabrication were moist cured for the entire period. Specimens tested 28-days after fabrication were moist cured for 7-days and then cured in air for an additional 21-days.

*** First visually observed cracking.

**** Maximum measured load.

TABLE 14—FLEXURAL CONSTANTS FROM STRESS VERSUS STRAIN TESTS

Mixture	Spec. No.	Cylinder Strength* psi (MPa)	$k_1 k_3$	k_3	$\frac{k_2}{k_1 k_3}$	ϵ_u	k_1	k_2	Modulus of Elasticity	
									By Compress-ometer Test ksi (MPa)	By Stress-Strain Curve ksi (MPa)
LWC1	1	8540 (58.9)	0.52	0.98	0.67	0.0028	0.53	0.35	3050 (21030)	3440 (23719)
	2	9300 (64.1)	0.54	0.94	0.63	0.0031	0.57	0.34	3310 (22823)	3420 (23581)
	3	9040 (62.3)	0.55	1.00	0.60	0.0030	0.55	0.33	3380 (23306)	3400 (23443)
LWC2	1	9080 (62.6)	0.55	0.97	0.60	0.0033	0.56	0.33	3360 (23168)	3100 (21375)
	2**	9150 (63.1)	--	--	--	--	--	--	3230 (22271)	-- ---
	3	9320 (64.3)	0.52	0.96	0.65	0.0033	0.54	0.34	3290 (22685)	3170 (21858)
HSLWC	1	10390 (71.6)	0.57	1.02	0.65	0.0035	0.56	0.37	3990 (27512)	3850 (26546)
	2	10700 (73.8)	0.54	0.96	0.67	0.0035	0.56	0.36	4020 (27718)	3700 (25512)
	3	10620 (73.2)	0.57	1.06	0.60	0.0026	0.54	0.34	4170 (28753)	4670 (32200)

* Average of compressive strengths measured on three 6 by 12 in. (150 by 300 mm) cylinders cured with the C-shaped test specimen.

** Specimen cracked in the test zone due to malfunctioning of the loading equipment. The results were disregarded.

TABLE 15—CALCULATION OF BOND DEVELOPMENT LENGTH

1. Basic Development Length (ACI 318-89, Section 12.2.2)

$$\ell_{db} = 0.04 A_b f_y / \sqrt{f'_c}$$

but not less than $0.03 d_b f_y / \sqrt{f'_c}$

for $f_y = 60,000$ psi (414 MPa)

$A_b = 0.44$ sq in. (284 sq mm)

$d_b = 0.75$ in. (19 mm)

$f'_c = 7,000$ psi (48 MPa)

$\ell_{db} = 12.6$ in. (320 mm)

2. Modification Factor for Sanded Lightweight Concrete
(ACI 318-89, Section 12.2.4.2)

$$\ell_d = 1.3 \times 12.6 \text{ in. (320 mm)} = 16.4 \text{ in. (417 mm)}$$

Use 16 in. (406 mm)

3. Modification for Bar Spacing (ACI 318-89, Section 12.2.3.4)

$$\ell_d = 0.8 \times 1.3 \times 12.6 \text{ in. (320 mm)} = 13.1 \text{ in. (333 mm)}$$

Use 13 in. (330 mm) -- Meets minimum length requirement for design.

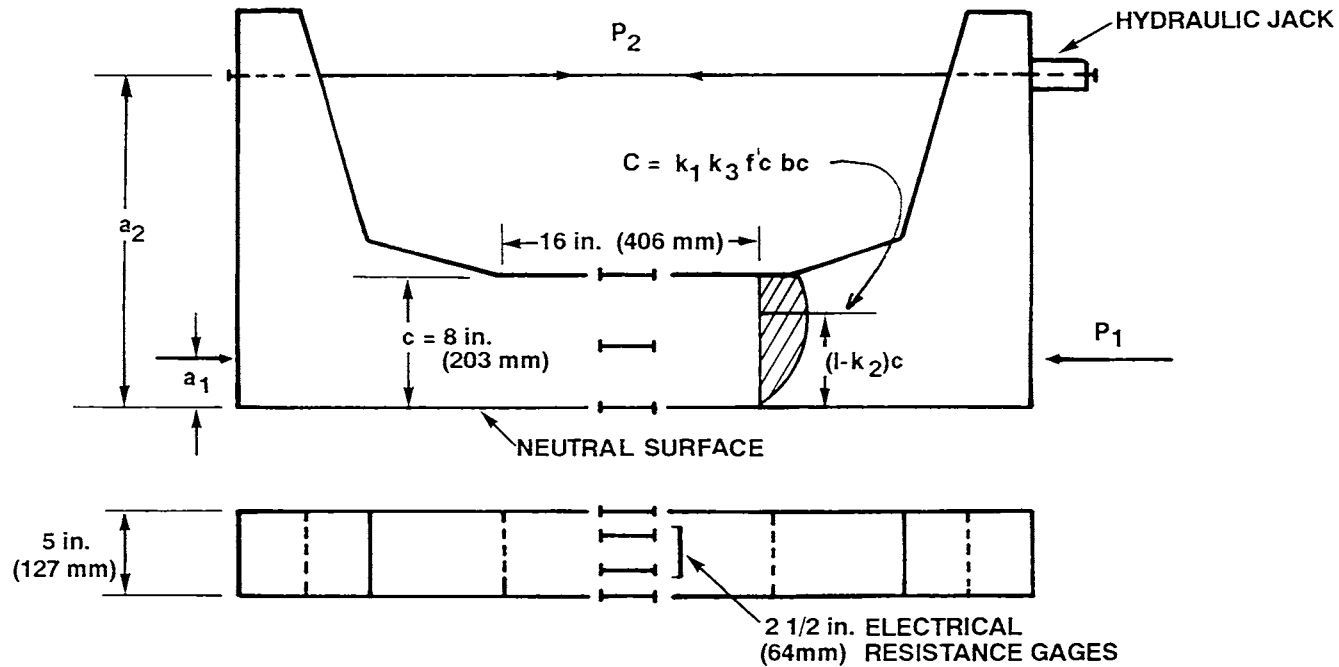


Fig. 1—C-shaped specimen for stress versus strain tests

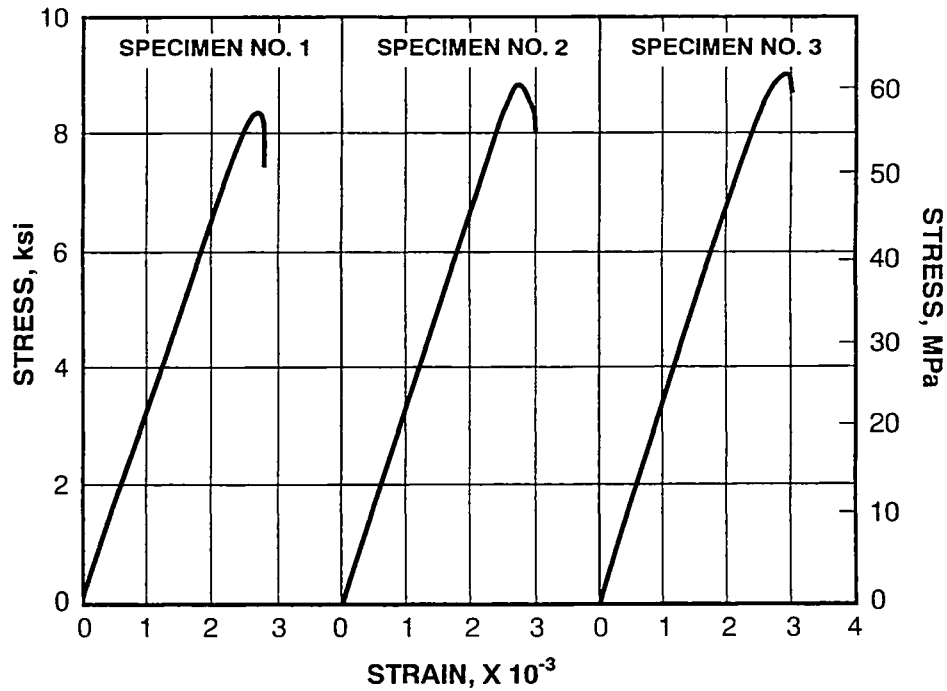


Fig. 2—Stress versus strain curves for LWC1