TABLE 8.14
MAXIMUM BRACING (LATERAL) CAPACITY OF TIMBER STUMPS

Height of stump		Maximum	bracing capac	city of timber	stumps, kN			
(E) above footing	Nominal unseasoned size of stumps, mm × mm							
mm	100 × 100	125 × 125	150 × 150	175 × 175	200 × 200	250 × 250		
200	19	37	50	50	50	50		
400	9.6	19	32	50	50	50		
600	6.4	12	22	34	50	50		
800	2.8	6.9	14	26	38	50		
1000	1.4	3.5	7.3	13	23	50		
1200	0.8	2.0	4.2	7.8	13	33		
1400	0.5	1.3	2.7	4.9	8.4	20		
1600	0.4	0.9	1.8	3.3	5.6	14		
1800	0.2	0.6	1.3	2.3	4	10		

NOTE: The following round timber stump sizes may be used in lieu of the square sizes given above:

- (a)  $100 \text{ mm} \times 100 \text{ mm} 125 \text{ mm diameter}$ .
- (b) 125 mm × 125 mm—150 mm diameter.
- (c)  $150 \text{ mm} \times 150 \text{ mm} 175 \text{ mm diameter}$ .
- (d) 175 mm × 175 mm—200 mm diameter.
- (e)  $200 \text{ mm} \times 200 \text{ mm}$ —225 mm diameter.
- (f)  $250 \text{ mm} \times 250 \text{ mm}$ —275 mm diameter.

## **8.3.5.7** Bracing columns

Timber, steel or concrete posts or columns placed into concrete footings may be used for transferring racking forces to the foundation. The horizontal load can be resisted by adding the capacity of individual stumps to resist the total force. Individual load capacities and details of columns or posts are given in Table 8.15 and Figure 8.4.

Where the column capacity is not adequate to resist the lateral load, additional bracing or cross-bracing shall be used.

All bracing shall be fixed to the floor or footing below and the floor above to enable the transfer of the full bracing capacity of the bracing system.

Steel columns over 900 mm above the ground shall not be used for bracing, unless incorporated in a bracing set.

Footing plan size and depth, as given in Table 8.15, shall apply to soil classifications A, S and M only.

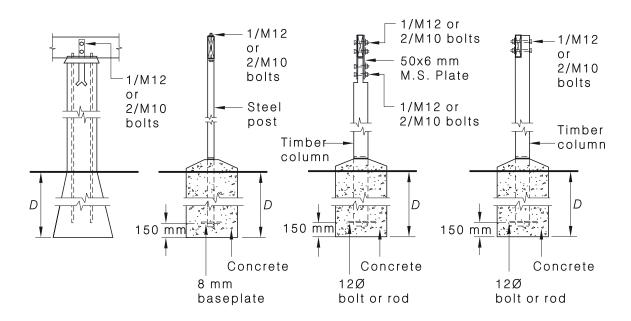
Bracing systems for other soil classifications, materials or sizes shall be designed in accordance with engineering principles.

TABLE 8.15
COLUMN BRACING CAPACITY

Height of	Column details				Footing		
column above ground	Concrete and masonry		Timber	Steel	plan size or	Footing depth (D)	Bracing capacity
	Plan size	Reinforce-	diameter	Steel	diameter	depen (2)	cupacity
mm	mm	ment	mm	mm	mm	mm	kN
600 or less	M200 × 200	1-Y12	125	$76 \times 76 \times 3.2$	$350 \times 350$	900	6
601 to 900	$M200 \times 200$	1-Y12	150	$76 \times 76 \times 4.0$	$350 \times 350$	900	4.5
901 to 1800	C200 × 200 M200 × 400 M300 × 300	4-R10	200	_	350 × 350	900	3
1801 to 2400	C200 × 200 M200 × 400 M300 × 300	4-Y12	225	_	400 × 400	900	3
2401 to 3000	C250 × 250 M200 × 400 M300 × 300	4-Y12	250	_	600 × 600	900	2.3

#### NOTES:

- 1 C = reinforced concrete column; M = reinforced concrete masonry.
- 2 Footing depth may be reduced to 600 mm when enclosed by a minimum of 100 mm thick concrete slab cast on the ground and of a minimum size of 6 m<sup>2</sup>.
- 3 For concrete and masonry columns and walls, see AS 3600 and AS 3700, respectively.
- 4 For bearer tie-down, see Section 9.



No-fines concrete shall be used for external hardwood columns

NOTE: For guidance on durability, see Appendix B.

FIGURE 8.4 CONCRETE, MASONRY AND STEEL BRACING COLUMNS

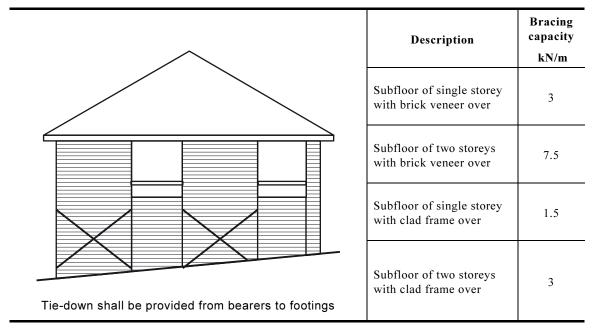
### **8.3.5.8** *Unreinforced masonry bracing*

Unreinforced masonry walls may be used to transfer racking forces in the subfloor region. The walls shall be a minimum of 90 mm thick, and engaged-piers shall be regularly spaced. All brickwork shall comply with AS 3700 or the Building Code of Australia.

Table 8.16 gives the capacity of masonry walls in the subfloor region only. The description of single- or two-storey, brick veneer or clad frame refers to the construction above the unreinforced masonry bracing wall under consideration. The bracing capacity of subfloor masonry is not applicable in regions where there are no walls above (for example, under verandah roofs, decks or similar structures).

The total minimum length of unreinforced masonry bracing walls in any full length of wall shall be 3000 mm with the minimum length of individual panels in the wall not less than 900 mm. The bracing capacities given in Table 8.16 are not applicable to stand-alone panels of masonry less than 3000 mm.

TABLE 8.16
UNREINFORCED MASONRY BRACING CAPACITY



**8.3.5.9** Spacing of bracing in the lower storey of two-storey construction or the subfloor of single- or two-storey construction

Bracing in the subfloor or lower storey of two-storey construction shall be evenly distributed. The maximum distance between bracing sets, stumps, piers, wall or posts, and similar constructions, under a strip or sheet timber floor system shall be as follows:

- (a) For wind classification C1, 14 000 mm if the minimum width of floor is 4800 mm.
- (b) For wind classification C2, 14 000 mm if the minimum width of floor is 6000 mm.
- (c) For wind classification C3, 11 500 mm if the minimum width of floor is 6000 mm.

If the width of the floor is less than as given above, the spacing of bracing shall be in accordance with Clause 8.3.6.7, where the width of the floor is considered as the ceiling depth.

NOTE: The minimum width of the floor is measured parallel to the direction of wind under consideration.

# 8.3.6 Wall bracing

#### **8.3.6.1** *General*

Walls shall be permanently braced to resist horizontal racking forces applied to the building. Wall bracing shall be designed to resist racking forces equal to or greater than the forces calculated from Clause 8.3.4.

The total capacity of bracing walls shall be the sum of the bracing capacities of individual walls. See Table 8.18 for the capacity of structural bracing walls, and see Section 9 for fixing requirements.

NOTE: The nail spacings given in Table 8.18 are nominal maximum spacings.

## **8.3.6.2** Nominal wall bracing

Nominal wall bracing is wall framing lined with sheet materials such as plywood, plasterboard, fibre cement, hardboard, or similar materials, with the wall frames nominally fixed to the floor and the roof or ceiling frame.

The maximum amount that can be resisted by nominal wall bracing is 50% of the total racking forces determined from Clause 8.3.4. Nominal wall bracing shall be evenly distributed throughout the building. If this is not the case, the contribution of nominal bracing shall be ignored.

The minimum length of nominal bracing walls shall be 450 mm.

The bracing capacity of nominal bracing is specified in Table 8.17.

TABLE 8.17
NOMINAL SHEET BRACING WALLS

Method	Bracing capacity, kN/m
Sheeted one side only	0.45
Sheeted two sides	0.75

#### **8.3.6.3** Structural wall bracing

Structural wall bracing is purpose-fitted bracing, being either sheet or cross-timber or steel bracing. Table 8.18 gives the specific capacity for each metre length of various structural bracing types.

### NOTES:

- Nominal bracing cannot contribute to bracing resistance where it occurs in the same section of wall as structural bracing, such as where plasterboard lining is fixed over a structural brace.
- 2 Where applicable, reference to top plate in Table 8.18 may also apply to ring beam.

For sheet-braced walls, the sheeting shall be continuous from the top plate or ring beam to the bottom plate with any horizontal sheet joins made over nogging with fixings the same as required for top and bottom plates.

Unless otherwise specified, sheet-bracing walls shall be a minimum of 900 mm wide to satisfy the requirements of their nominated ratings.

The capacity of sheet bracing given in bracing types (g) to (n) in Table 8.18 is based on fixing the sheeting to framing having a minimum joint strength group of J4 or JD4. If JD5 is used, the bracing capacity given bracing types (g) to (k) in Table 8.18 shall be reduced by 12.5%, and in bracing types (l) to (n), by 16%.

# NOTES:

- 1 Joint groups for commonly available timbers are given in Clause 9.6.5 and Appendix G.
- For wall heights greater than 2700 mm, the values in Table 8.18 may be proportioned downward relative to the wall heights. For example, for a wall height of 3600 mm multiply the values in the table by 2700/3600 = 0.75 (see Clause 8.3.6.4).

TABLE 8.18
STRUCTURAL WALL BRACING (MAXIMUM WALL HEIGHT 2.7 m)

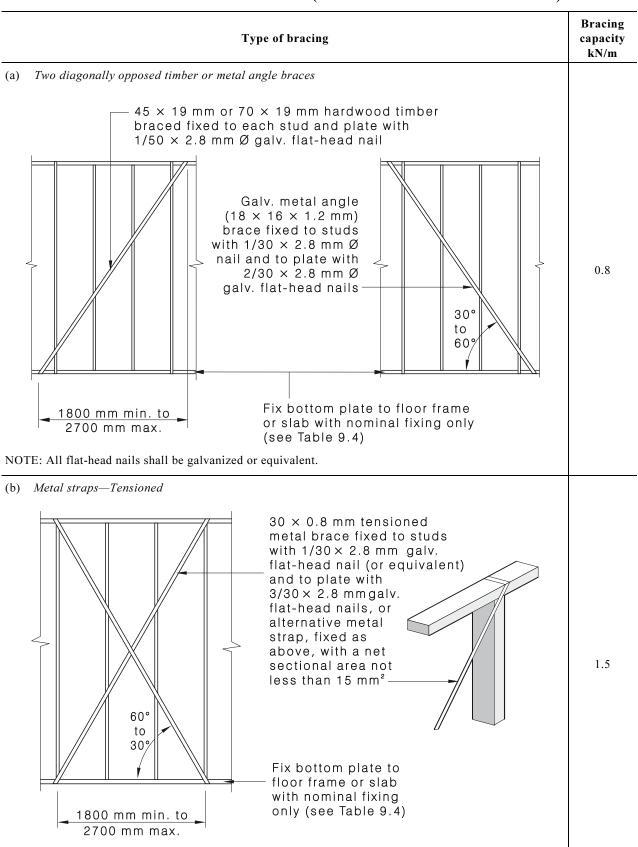
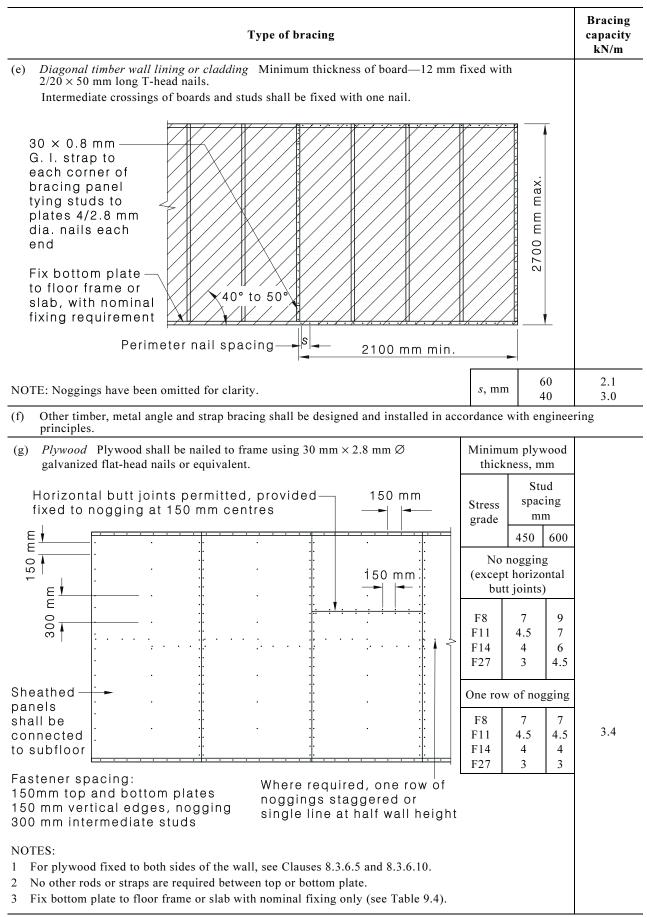


TABLE 8.18 (continued)

# **Bracing** Type of bracing capacity kN/m (c) Timber and metal angle braces The maximum depth of a notch or saw-cut shall not exceed 20 mm. Saw-cuts studs shall be designed as notched. $2/50 \times 2.8 \text{ mm } \emptyset \text{ nails for timber brace}$ Min. $75 \times 15$ mm F8 brace or or $2/30 \times 2.8$ mm Ø nails for metal brace, metal angle of min. nominal section $20 \times 18 \times 1.2 \text{ mm}$ to each stud and plate (See Detail 1) No end splits allowed; drill if necessary 1.5 (See Detail 1) (See Detail 1) 1800 mm min. to 2700 mm max. Fix bottom plate to floor $\underline{\text{Detail 1}}$ : 30 $\times$ 0.8 mm galv. metal strap looped over plate frame or slab and fixed to stud with $3/30 \times 2.8 \text{ mm } \emptyset$ galv. flat-head with nominal nails (or equivalent) to each end. Alternatively, provide fixing only single straps to both sides, with 3 nails per strap end, (see Table 9.4) or equivalent anchors or other fasteners. (d) Metal straps—Tensioned—With stud straps $30 \times 0.8$ mm galv. metal strap looped over plate and fixed to stud with $4/30 \times 2.8 \text{ mm } \emptyset \text{ galv.}$ flat-head nails (or equivalent) to each end. Alternatively, provide single straps to both sides, with 4 nails per strap end, or equivalent anchors or other fasteners 3.0 $30 \times 0.8$ mm tensioned metal strap fixed to studs with one $30 \times 2.8 \text{ mm } \emptyset \text{ galv. flat-head}$ nail (or equivalent) and to plates with $4/30 \times 2.8 \text{ mm } \emptyset$ 30° to galv. flat-head nails, or 60° alternative metal strap, fixed as above, with a net sectional area not less than 21 mm2 1800 mm min. to Fix bottom plate to floor frame or 2700 mm max. slab, with nominal fixing requirement

TABLE 8.18 (continued)



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TABLE 8.18 (continued)

	Type of bracing				Bracing capacity kN/m
nai	Plywood Plywood shall be nailed to frame using 30 × 2.8 Ø galvanized flat-head nails or equivalent.  For Method A, M12 rods shall be used at each end of sheathed section top plate to bottom plate/floor frame. Method B has no rods but sheathing shall be nailed to top and bottom plates and any horizontal joints at 50 mm centres.  izontal butt joints are permitted, provided fixed to nogging at s = 150 mm centres for hod A, or s = 50 mm centres for Method B	thick Stress grade F8 F11 F14 F27 Fastener Top and plate: — Method Vertical e Intermedistuds Fixing plate to f	od A od B dges ate g of botto loor fran slab A: M12 r	150 50 150 300	Method A 6.4 Method B 6.0
plat	hod A only: M12 rod top to bottom Sheathed panels shall be each end of sheathed section connected to subfloor  E: For plywood fixed to both sides of the wall, see Clauses 8.3.6.5 and 10.  Plywood Plywood shall be nailed to frame using 30 × 2.8 mm Ø galvanized flat-head nails or equivalent.	13 kN ca connection 1200 mm Method I capacity at each e intermed max. 120 centres	pacity on at may centres 3: A 13 k connecti nd and iately at	aN on ood	
	rizontal butt joints are permitted, provided  50 mm  d to nogging at 50 mm centres  50 mm  50 mm	Stress grade No (excep but	Students Stu	d ng 600	
E T		F11 F11	4.5 7.0	7.0	7.5 8.7
100			ner spacin		8.7
		Top and plate	bottom	50	
1440	rod top to bottom. Fix bottom plate to floor forms and to be seen as a large with	Vertical	edges	100	
plat she	rod top to bottom —Fix bottom plate to floor frame or slab with e each end of M12 rods as shown, plus a 13 kN capacity athed section connection at max. 600 mm centres E: For plywood fixed to both sides of the wall, see Clauses 8.3.6.5 and 10.		iate	100	

TABLE 8.18 (continued)

Type of bracing			Bracing capacity kN/m
<ul> <li>(j) Decorative plywood—Nailed Decorative plywood shall be nailed to frame using min. 40 mm × 2.5 mm Ø bullet-head nails.</li> <li>The depth of groove shall not exceed one-third the nominal thickness.</li> </ul>	Minimum nominal thickness of decorative structural plywood, mm		
Skew-nailed in groove and punched on edge of plywood sheet	Stress grade	Stud spacing mm (600 max.)	
	F11	6	
	Fastener spacing mm		2.1
200 mm	Top and be	ottom 100	
	Vertical ed	dges 100	
NOTE: Fix bottom plate to floor frame or slab with nominal fixing only	Intermedia	ate studs 200	
<ul> <li>(k) Decorative plywood—Glued and nailed Decorative plywood shall be nailed to frame using min. 40 × 2.5 mm Ø bullet-head nails. Continuous 6 mm bead of elastomeric adhesive to studs and plates. Double 6 mm glue bead where plywood sheets butt together on a common stud.         The depth of groove shall not exceed one-third the nominal thickness.     </li> </ul>	thickness	m nominal of decorative plywood, mm  Stud spacing mm	
200 mm  Double 6 mm glue bead where plywood sheets butt together on a common stud		(600 max.)	
	F11	6	
200 mm		er spacing mm	5.3
	Top and be	ottom 200	
	Vertical ed	dges 200	
Skew-nailed in groove and punched on edge of plywood sheet  NOTE: Fix bottom plate to floor frame or slab with a 13 kN capacity connection	Intermedia	ate studs 200	
at each end of braced panel and at max. 1200 mm centres.			

TABLE8.18 (continued)

Type of bracing			Bracing capacity kN/m	
(1) Hardboard Type A Hardboard shall comply with AS/NZS 1859.4. Minimum hardboard thickness 4.8 mm				
galvanized flat-head nails or equivalent.  Nails shall be located a minimum of 10 mm from the vertical edges and	Fastener spacing	, mm		
15 mm from the top and bottom edges. Maximum stud spacing = 600 mm. Bracing panel minimum width = 900 mm.	Top and bottom plates	80		
——   <del>-</del> 80 mm	Vertical edges and nogging	150		
E	Intermediate studs	300		
Shall be lined	Fixing of bottom to floor frame of	Type A 3.4		
1 150 mm   Willi gypsulli   /	Type A:		3.4	
Fixing bottom plate to floor frame or slab wit				
Fix bottom  plate to floor  frame or slab	requirement (see Table 9.4).			
(m) Hardboard Types B and C Hardboard shall comply with AS/NZS 1859.4. Hardboard shall be nailed to frame using minimum $30 \times 2.8 \text{ mm } \emptyset$	Minimum hard thickness 4.8			
galvanized flat-head nails or equivalent.	Fastener spacing	, mm		
Nails shall be located a minimum of 10 mm from the vertical edges and 15 mm from the top and bottom edges. Maximum stud spacing = 600 mm. Bracing panel minimum width = 900 mm.	Top and bottom plates	40		
40 mm Type C only: M12 rod at each end and	Vertical edges and nogging	150		
staggered — max. 1800 mm centres in between	Intermediate stud	ls 300		
At least one	Fixing of bottom to floor frame or			
E   · · · · · · · · · · · · · · · · · ·	Type B:		Type B 6.0	
shall be lined.  with gypsum.  plaster board.  or equivalent.	Fix bottom plate floor frame or sla M10 bolts each e intermediately at 1200 mm centres	ab with and and max.	Type C 9.0	
E	Type C:			
3008	M12 rods at each end and intermediately at max. 1800 mm centres.			
т т	NOTE: Bolt/rod	washer		
	sizes as per Tabl			