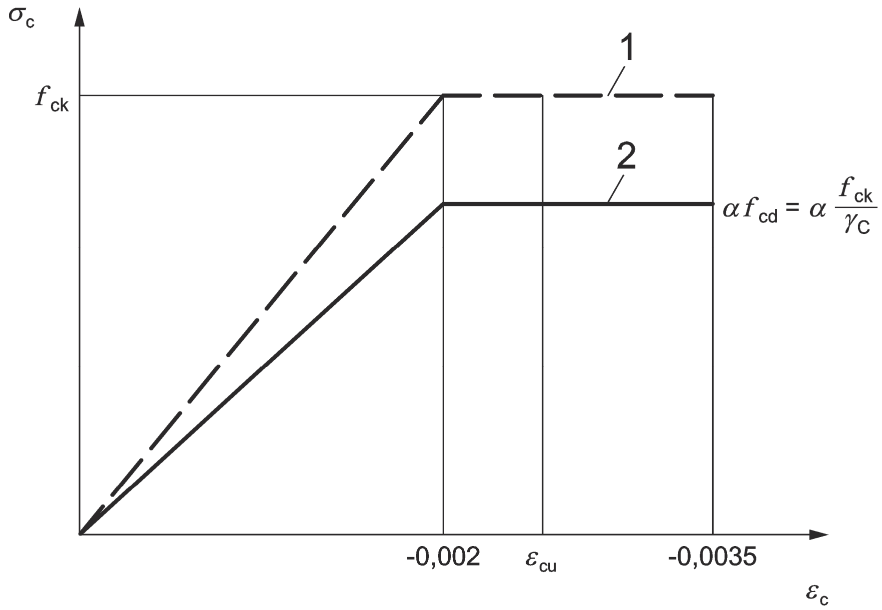
NOTE 1 The value of γ_C for use in a country can be found in its national application document. The recommended values for use are given in Table C.1.

(2)P Giving allowance for long term effects on the compressive strength of LAC the design strength is multiplied by the coefficient α.

NOTE 2 The value of α for use in a country can be found in the national application document. The recommended value of α is 0,85. In cases where the compression zone decreases in width in the direction of the extreme compression fibre α is recommended to be reduced to 0,80.



Key

1 idealized diagram
2 design diagram

Figure A.1 — Bi-linear stress-strain diagram for LAC in compression for cross-sectional design

A.4.3 Stress-strain diagram for reinforcement steel

(1)P The design value f_{yd} of the yield strength of reinforcement steel is defined by:

$$f_{yd} = f_{yk}/\gamma_S \qquad \text{(A.3)}$$

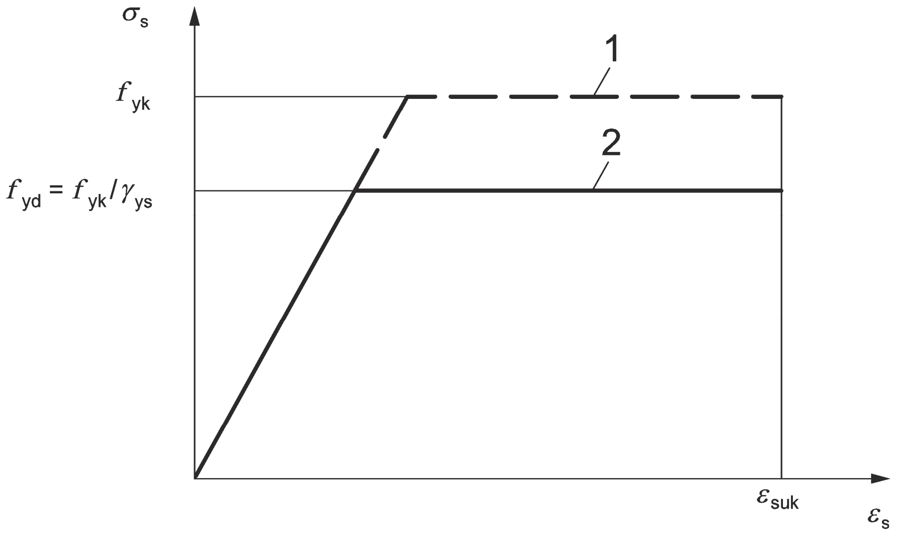
where

f_{yk} is the characteristic yield strength of reinforcement steel;

γ_S is the partial safety factor for reinforcement steel.

NOTE The value of γ_S for use in a country can be found in its national application document. Recommended values are given in Table C.1.

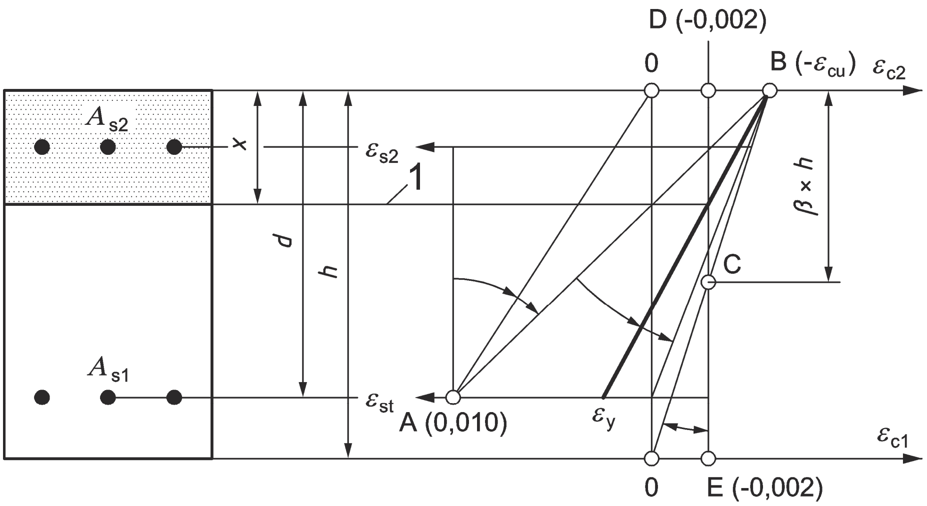
(2) The design stress-strain diagram for reinforcement steel is given in Figure A.2. Other established stress-strain diagrams may be used. E_s is the modulus of elasticity of reinforcement steel (e.g. 2 x 10^5 MPa).



Key

1 idealized diagram
2 design diagram

Figure A.2 — Design stress-strain diagram for reinforcement steel



Key

1 neutral axis

The value of β is equal to (ε_{cu} - 0,002)/ε_{cu}, where ε_{cu} is calculated from Equation (A.1).

Figure A.3 — Possible strain diagrams in the ultimate limit state

A.5 Ultimate limit state induced by shear

A.5.1 Shear design for components predominantly under transverse load not requiring shear reinforcement

(1)P The following conditions shall be met:

$$V_{Ed} < V_{Rd1} \tag{A.4a}$$

$$V_{Ed} < V_{Rd2} \tag{A.4b}$$

where

V_{Ed} is the design shear force in the section;

V_{Rd1} is the design shear resistance of a component without shear reinforcement;

V_{Rd2} is the design shear crushing resistance (maximum shear force that can be carried without crushing of the notional concrete compressive struts), which is given by Equation (A.14).

The design shear force does not need to be checked at a distance of less than $d/2$ from the support. A reduction of concentrated loads near the support is not allowed.

NOTE 1 The value of V_{Rd1} can be estimated by Equation (A.5) or Equation (A.10) as specified in the national application document.

(2) The design shear resistance V_{Rd1} may be estimated as:

$$V_{Rd1} = C_{Rd} k \cdot \eta_1 (100\ \rho_1 f_{ck})^{1/3} b_w d \qquad \text{(A.5a)}$$

with a minimum of

$$V_{Rd1} = v_{mind}\ b_w d \qquad \text{(A.5b)}$$

where

C_{Rd} is a parameter for shear capacity;

k is a factor for the shear strength, see Equation (A.8);

η_1 is a coefficient according to Equation (A.9);

ρ_1 is the reinforcement ratio $\rho_1 = A_{s1}/(b_w\ d) \leqq 0{,}02$;

f_{ck} is the characteristic compressive strength of LAC, in megapascals;

A_{s1} is the cross-sectional area of the longitudinal reinforcement, anchored as described in A.9, in square millimetres;

b_w is the minimum width of the cross-section over the effective depth, in millimetres;

d is the effective depth of the cross-section, in millimetres;

v_{mind} is the minimum shear strength of LAC, in megapascals.

NOTE 2 The values of C_{rd} and v_{mind} can be found in the national application document. Recommended values are:

$$C_{Rd} = 0{,}145 / \gamma_C \qquad \text{(A.6)}$$

$$v_{mind} = 0{,}03 \cdot k^{3/2} \cdot f_{ck}^{1/2} \qquad \text{(A.7)}$$

$$k = 1 + (200/d)^{1/2} \leq 2{,}0 \qquad \text{(A.8)}$$

$$\eta_1 = 0{,}40 + 0{,}60 \rho / 2\ 200 \qquad \text{(A.9)}$$

where

γ_C is the partial safety factor for LAC;

f_{ck} is the characteristic compressive strength of LAC, in megapascals;

ρ is the dry density of LAC, in kilograms per cubic metre.

(3) The design shear resistance V_{Rd1} may be estimated as:

$$V_{Rd1} = \tau_{Rd}\ k\ (1{,}2 + 4 \ldots \qquad \text{(A.10)}$$

$$\tau_{Rd} = \tau_{Rk} / \gamma_C \tag{A.11}$$

$$\tau_{Rk} = 0{,}125\, f_{t,flk} \tag{A.12}$$

$$k = 1{,}6 - d/1000 \geq 1 \tag{A.13}$$

where

τ_{Rk} is the characteristic shear strength of LAC, in megapascals;

k is a factor for the shear strength, see Equation (A.13);

ρ_1 is the reinforcement ratio $\rho_1 = A_{s1}/(b_w\, d) \leqq 0{,}02$;

τ_{Rd} is the design shear strength of LAC, in megapascals;

γ_C is the partial safety factor for LAC;

A_{s1} is the cross-sectional area of the longitudinal reinforcement, anchored as described in A.9, in square millimetres;

b_w is the minimum width of the cross-section over the effective depth, in millimetres;

d is the effective depth of the cross-section, in millimetres;

$f_{t,flk}$ is the characteristic flexural strength of LAC (see 4.2.4), in megapascals.

(4) The design shear crushing resistance V_{Rd2} is given by:

$$V_{Rd2} = 0{,}5\ \eta_1\, b_w\, z\ \nu\, f_{ck} / \gamma_C \tag{A.14}$$

where

η_1 is a coefficient according to Equation (A.9);

b_w is the minimum width of the cross-section over the effective depth, in millimetres;

z is the height of the shear zone, which is taken as the distance between the centres of the compression zone and the longitudinal reinforcement (lever arm of internal forces), in metres. This may be set to $0{,}9d$ in normal designs, but not more than the height (h_w) of the shear reinforcement;

d is the effective depth of the cross-section, in metres;

h_w is the height of the shear reinforcement, in metres;

ν is the efficiency factor, which shall be taken as 0,6;

f_{ck} is the characteristic compressive strength of LAC, in megapascals;

γ_C is the partial safety factor for LAC.

A.5.2 Shear design for components predominantly under transverse load requiring shear reinforcement

(1)P The following conditions shall be met:

$$V_{Ed} < V_{Rd3} \tag{A.15a}$$

$$V_{Ed} < V_{Rd2} \tag{A.15b}$$

where

V_{Ed} is the design shear force in the section;

V_{Rd2} is the design shear crushing resistance (maximum shear force that can be carried without crushing of the notional concrete compressive struts), which is given by Equation (A.14);

V_{Rd3} is the design shear resistance of the component with shear reinforcement.

The design shear force does not need to be checked at a distance of less than $d/2$ from the support. A reduction of concentrated loads near the support is not allowed.

NOTE 1 The value of V_{Rd3} can be estimated by Equation (A.16) or Equation (A.17) as specified in the national application document.

(2)P For beams with vertical shear reinforcement (ladder-type), the shear resistance V_{Rd3} is

$$V_{Rd3} = (A_{sw}/s) \cdot z \cdot f_{ywd} \cdot \cot\theta \qquad \text{(A.16)}$$

where

A_{sw} is the cross-sectional area of shear reinforcement, in square millimetres;

s is the spacing of the shear reinforcement bars, in metres;

z is the height of the shear zone, which is taken as the distance between the centres of the compression zone and the longitudinal reinforcement (lever arm of internal forces), in metres. This may be set to 0,9d in normal designs, but not more than h_w;

h_w is the height of the shear reinforcement, in metres;

f_{ywd} is the design yield strength of the shear reinforcement ($f_{ywd} = f_{ywk}/\gamma_S$), in megapascals;

f_{ywk} is the characteristic value of the yield strength of the shear reinforcement, in megapascals;

γ_S is the partial safety factor for reinforcement steel;

θ is the angle of the compression struts.

The shear reinforcement shall fulfil the minimum requirements in (4).

The angle θ of the compression struts shall be limited.

NOTE 2 The limiting values of $\cot\theta$ can be found in the national application document. The recommended limits are $1 \leq \cot\theta \leq 2{,}5$.

(3)P For beams with vertical shear reinforcement (ladder-type), the shear resistance V_{Rd3} is

$$V_{Rd3} = V_{Rd1} + V_{wd} \qquad \text{(A.17)}$$

where V_{Rd1} is the design shear resistance of a component without shear reinforcement given by Equation (A.10) and V_{wd} is estimated by Equation (A.18) as

$$V_{wd} = 0{,}8\,(A_{sw}/s)\,z\,f_{ywd} \qquad \text{(A.18)}$$

where

A_{sw} is the area of shear reinforcement, in square millimetres;

s is the spacing of the shear reinforcement bars, in metres;

z is the height of the shear zone, which is taken as the distance between the centres of the compression zone and the longitudinal reinforcement (lever arm of internal forces), in metres. This may be set to 0,9d in normal designs, but not more than h_w;

d is the effective depth of the cross-section, in metres;

h_w is the height of the shear reinforcement, in metres;

f_{ywd} is the design yield strength of the shear reinforcement ($f_{ywd} = f_{ywk}/\gamma_S$), in megapascals;

f_{ywk} is the characteristic value of the yield strength of the shear reinforcement, in megapascals;

γ_S is the partial safety factor for reinforcement steel.

The shear reinforcement shall fulfil the minimum requirements in (4).

(4) The minimum shear reinforcement ratio ρ_w for beams shall be at least

$$\rho_{w,min} = 0{,}08 f_{ck}^{\frac{1}{2}} / f_{yk} \quad \text{(A.19)}$$

where

$$\rho_w = A_{sw} / (s \cdot b_w) \quad \text{(A.20)}$$

and where

f_{ck} is the characteristic compressive strength of LAC, in megapascals;

f_{yk} is the characteristic yield strength of the reinforcement steel, in megapascals;

A_{sw} is the cross-sectional area of shear reinforcement within length s, in square millimetres;

s is the spacing of the shear reinforcement measured along the longitudinal axis of the member, in millimetres;

b_w is the width of the web of the beam, in millimetres.

Using reinforcement steel with yield strength f_{yk} = 500 MPa will lead to the following minimum percentages of shear reinforcement shown in Table A.1:

Table A.1 — Minimum percentages of shear reinforcement $\rho_{w,min}$ for reinforcement steel with f_{yk}= 500 MPa

Characteristic compressive strength of LAC f_{ck} MPa	$\rho_{w,min}$ %
2	0,023
4	0,032
6	0,039
8	0,045
10	0,051
12	0,055
15	0,062
20	0,072
25	0,080

The shear reinforcement shall be vertical and welded to the longitudinal reinforcement. Welding and anchorage shall be verified according to A.9.

(5) The value of the characteristic value of the yield strength of the shear reinforcement f_{ywk} should be limited.

NOTE 3 The limiting value of f_{ywk} can be found in the national application document. The recommended value is $f_{ywk} \leq$ 400 MPa.

A.5.3 Shear design for components under predominantly longitudinal compression forces, e.g. walls and piers

(1)P Only the portions of the cross-section which remain without tensile stresses under the relevant load combination in the ultimate limit state are considered to be capable of resisting concrete stresses due to shear.

(2)P The design shear resistance is:

$$V_{Rd4} = \tau_{Rd}\, b_w\, x \,/\, 1{,}5 \qquad \text{(A.21)}$$

$$\tau_{Rd} = 0{,}125\, f_{t,flk} \,/\, \gamma_C \qquad \text{(A.22)}$$

where

τ_{Rd} is the basic shear strength of LAC, in megapascals;

b_w is the minimum width of the section in the compression zone, in millimetres;

x is the neutral axis depth $x \leq h$; calculated using first order theory, in millimetres;

$f_{t,flk}$ is the characteristic flexural strength of LAC (see 4.3.4), in megapascals;

h is the overall depth of the cross-section, in millimetres;

γ_C is the partial safety factor for LAC.

In cross-sections where the zone of decompression in the ultimate limit state extends further than to the centre of the cross-section it shall be verified that:

$$N_{Sd} \,/\, V_{Sd} \geq 2{,}0 \qquad \text{(A.23)}$$

where

N_{Sd} is the design axial compression force in the section;

V_{Sd} is the design shear force in the section.

A.6 Ultimate limit state induced by structural deformation (buckling)

A.6.1 General

(1)P When determining the loadbearing capacity of slender LAC components subjected to compression or combined bending and compression, account shall be taken of the effects of structural deformation and eccentricities occurring perpendicular to the plane of the components and their influence on the buckling of the components.

(2) Two methods are given in A.6.2 and A.6.3. They are both suitable for designing vertical loadbearing LAC components which can be classified as slender isolated columns or walls and are mainly loaded by a central or eccentric longitudinal load and possibly also by a transverse load (e.g. horizontal wind load, earth pressure).

(3)P The slenderness ratio S of the components shall not exceed the values indicated in Figure A.4,

where

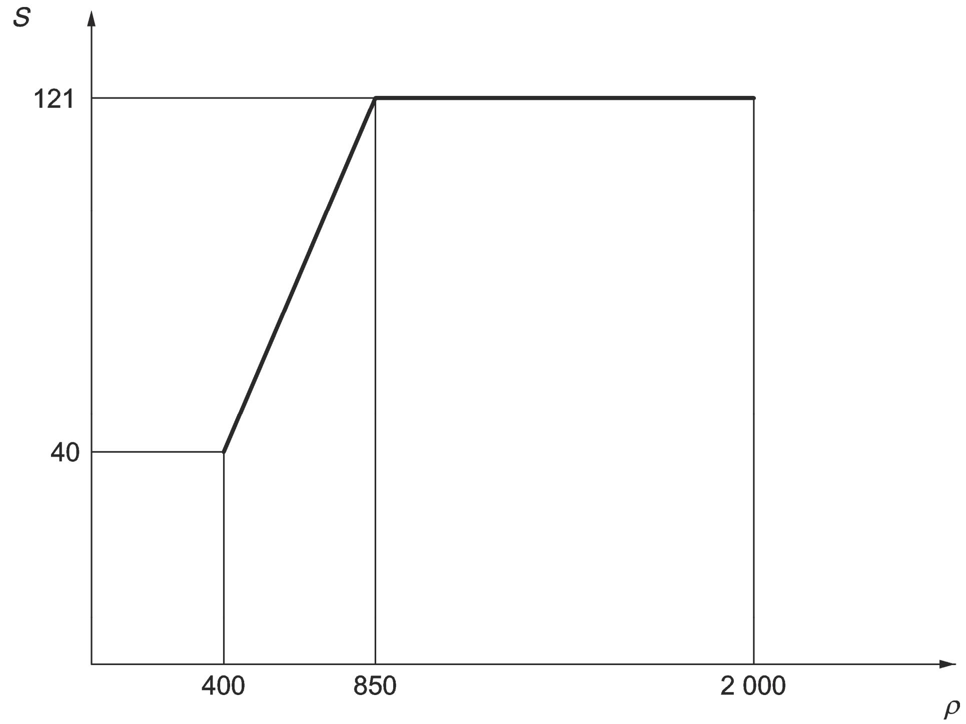
S is the slenderness ratio; $S = l_0/i_w \leq 121$;

l_0 is the buckling length of the component, in metres. The buckling length (effective height with respect to buckling) will depend on the support conditions, as shown in Table A.2;

i_w is the radius of inertia in the direction of the weak axis (i_w = 0,289h for rectangular solid cross-sections), in metres;

h is the overall thickness of the component, in metres;

ρ is the mean value of the dry density of the LAC, in kilograms per cubic metre.



Key

S slenderness ratio
ρ mean dry density of LAC

Figure A.4 — Limits of slenderness ratio S of loadbearing walls and piers

NOTE The use of the design methods according to the different methods of this subclause (A.6.2, A.6.3 or A.6.4) in a country is specified in the national application document. The national application document can limit the use of these components for different structural applications such that e.g. different minimum thicknesses are required or different maximum slenderness limits are specified for certain structural applications.

A.6.2 Method based on the Euler formula

(1)P The resisting design axial compression force N_{Rd} shall be determined as the loadbearing capacity of that part of the cross-section, which can be regarded as centrally loaded, i.e.:

$$N_{Rd} = k_s \cdot \alpha \cdot f_{cd} A_c \qquad \text{(A.24)}$$

where

k_s is the column factor according to Equation (A.25);

α is the long term factor according to A.4.2;

f_{cd} is the design value of the compressive strength of LAC;

A_c is the area of the compression zone of the cross-section;

$$k_s = \frac{1}{1 + \frac{f_{ck}}{E_{cm}\pi^2} \cdot \left(\frac{l_0}{i_c}\right)^2} \quad \text{(A.25)}$$

where

E_{cm} is the mean value of the modulus of elasticity of LAC according to 4.2.6;

f_{ck} is the characteristic compressive strength of LAC;

l_0 is the buckling length of the component equal to $\beta \cdot l_w$, where β depends on the support conditions as shown in Table A.2.

β is a coefficient for the determination of the buckling length (see Table A.2);

l_w is the height of the wall component;

i_c is the radius of inertia of the compression zone of the cross-section, i.e. $i_c = (I_c/A_c)^{½}$;

I_c is the moment of inertia of the compression zone of the cross-section;

A_c is the area of the compression zone of the cross-section.

For rectangular cross-sections or cross-sections with other shapes that can be converted into equivalent rectangular cross-sections the following may be used:

$$A_c = l_h (h - 2e_1) \quad \text{(A.26)}$$

$$i_c = (h - 2e_1)/\sqrt{12} \quad \text{(A.27)}$$

where

h is the design thickness of the component, which for a solid wall is equal to the thickness of the wall;

e_1 is the first order eccentricity perpendicular to the wall taken as the sum of the first order eccentricity of the longitudinal load in the middle third of the height of the wall (e_0) and the additional eccentricity of the longitudinal compression force due to geometrical imperfection (e_a). The geometrical imperfection shall be taken as 1/500 of the total component height.

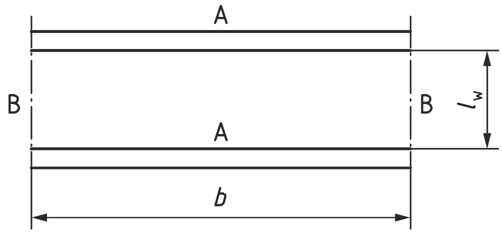
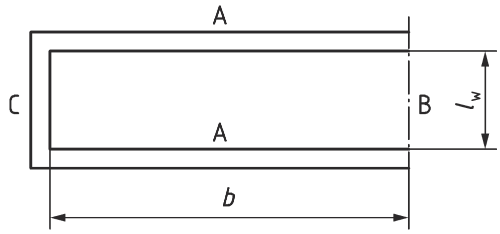
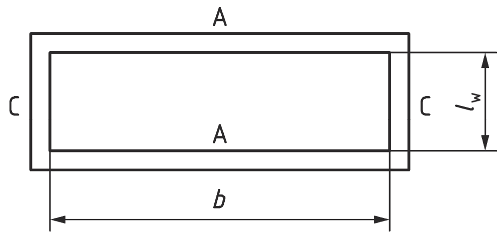
l_h is the effective horizontal length of the cross-section.

The cross-sections at the top and bottom of the wall need to be checked for crushing in the case of larger bending moments at the top or bottom, than the moment in the middle third of the wall. This is checked by using the Equation (A.24) with k_s=1,0.

NOTE 1 The national application document can limit the use of the components with respect to slenderness and loadbearing capacity.

(2) For the determination of the buckling length $l_0 = \beta \cdot l_w$ (coefficient β see Table A.2) a multi-sided support of a wall should be assumed only if the wall is erected without in-plane joints between the supporting walls.

Table A.2 — Coefficient β for the determination of the buckling length l_0 at different boundary conditions
(used in Equation (A. 25), $l_0 = \beta \cdot l_w$)

<table>
<tr><th>Lateral restraint</th><th>Sketch</th><th>Expression</th><th colspan="2">Factor β</th></tr>
<tr><td>Along two edges</td><td>
</td><td></td><td colspan="2">β =1,0 for any ratio of l_w/b</td></tr>
<tr><td rowspan="2">Along three edges</td><td rowspan="2">
</td><td rowspan="2">$$\beta = \frac{1}{1+\left(\frac{l_w}{3b}\right)^2}$$</td><td>b/l_w</td><td>β</td></tr>
<tr><td>0,2
0,4
0,6
0,8
1,0
1,5
2,0
5,0</td><td>0,26
0,59
0,76
0,85
0,90
0,95
0,97
1,00</td></tr>
<tr><td rowspan="2">Along four edges</td><td rowspan="2">
</td><td rowspan="2">If $b \geq l_w$
$$\beta = \frac{1}{1+\left(\frac{l_w}{b}\right)^2}$$
If $b < l_w$
$$\beta = \frac{b}{2l_w}$$</td><td>b/l_w</td><td>β</td></tr>
<tr><td>0,2
0,4
0,6
0,8
1,0
1,5
2,0
5,0</td><td>0,10
0,20
0,30
0,40
0,50
0,69
0,80
0,96</td></tr>
<tr><td>Key</td><td colspan="4">A… floor slab
B… free edge
C… transverse wall</td></tr>
</table>

NOTE 2 The information in Table A.2 assumes that the wall has no openings with a height exceeding 1/3 of the wall height l_w or with an area exceeding 1/10 of the wall area. In walls laterally restrained along 3 or 4 sides with openings exceeding these limits, the parts between the openings should be considered as laterally restrained along 2 sides only and be designed accordingly.

NOTE 3 The information in Table A.2 assumes that the wall is restrained sufficiently by transverse walls and that mechanical joints connect the restrained wall and the transverse walls. Requirements for transverse walls and mechanical joint need to be considered by the designer, just as guidance or rules can be found in national documents or in EN 1992-1-1.

A.6.3 Modified model column method

A.6.3.1 General

(1)P When determining the loadbearing capacity account shall be taken of the effect of slenderness and of any moments and eccentricities occurring and their influence on the buckling of wall components.

(2)P In the case of eccentric compression in the plane of the structure, components shall be analysed for buckling about the weak axis.